



Savannah River  
Remediation

# Life Estimation of High Level Waste Tank Steel for F-Tank Farm Closure Performance Assessment

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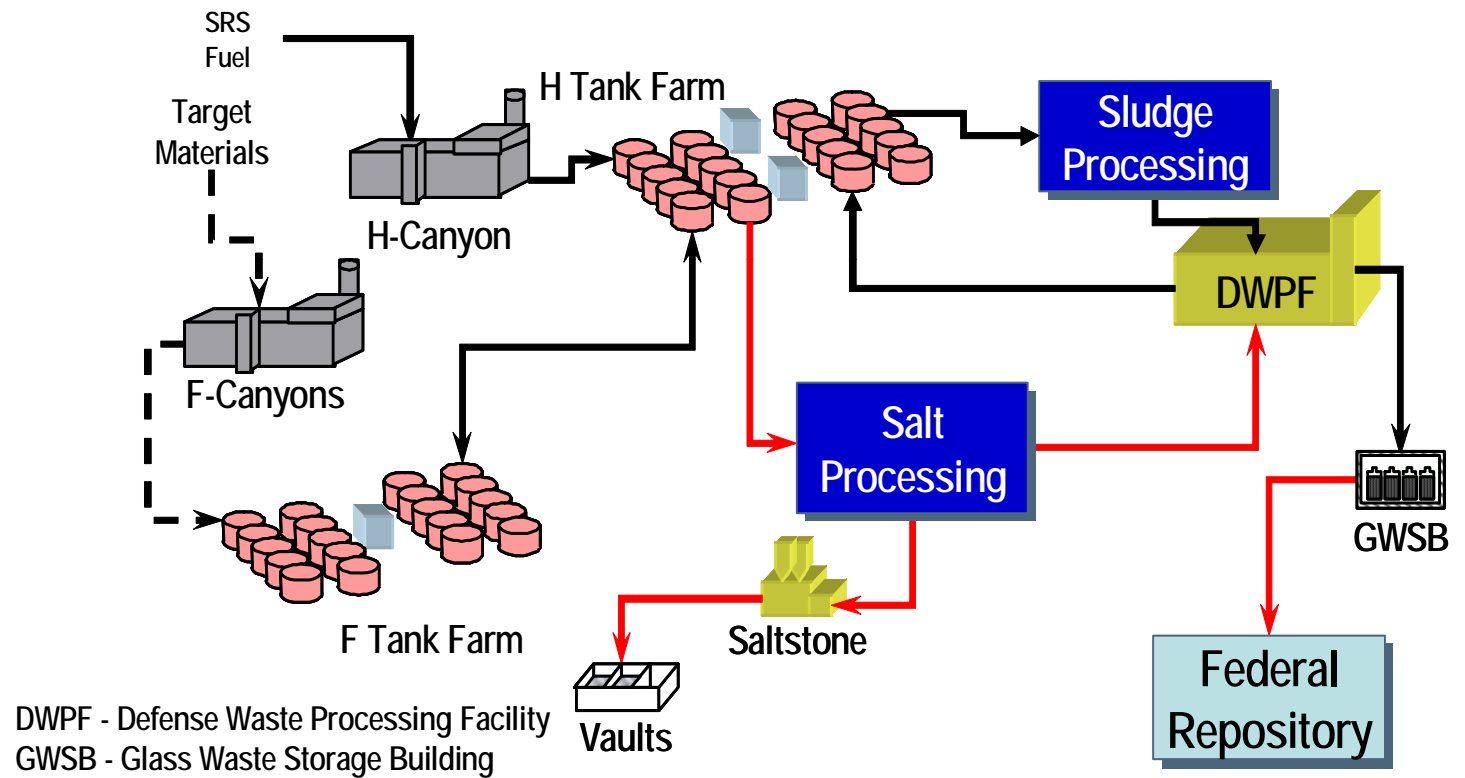
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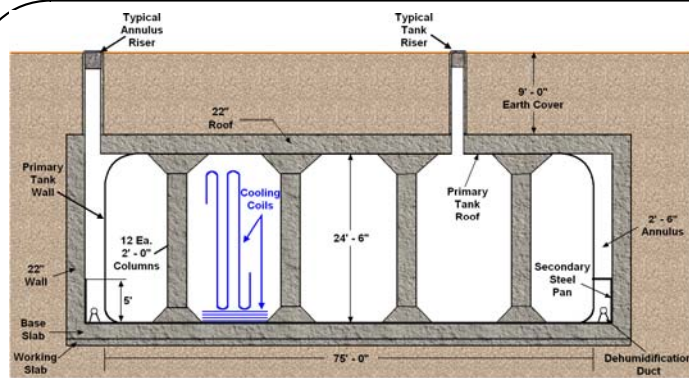
# Outline

- Savannah River Site liquid waste operations
- Performance assessment for tank closure
- Tank life estimation technical approach
- Results
- Recommendations

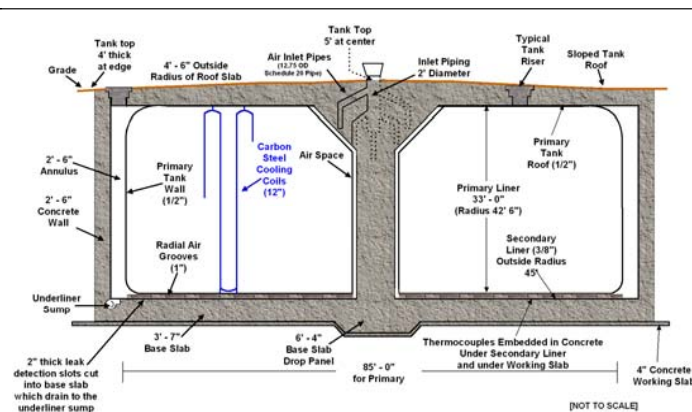
# Savannah River Site Liquid Waste System



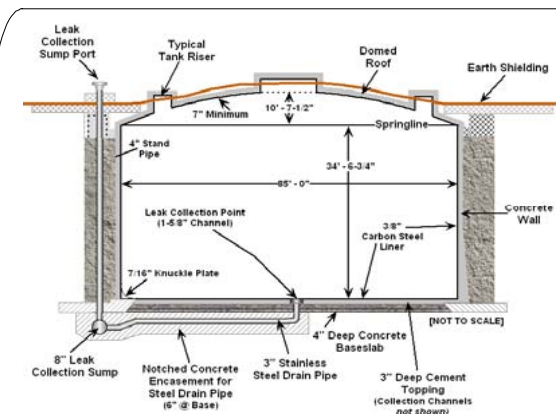
# F-Tank Farm Liquid Waste Tanks



- Type I Tanks
- Vintage 1950s
- Built of ASTM A285, Grade B
- Non-stress relieved
- Partial secondary containment
- 0.5-in plate construction



- Type III/IIIA Tanks
- Vintage 1970s-1980s
- Built of ASTM A537-CI.1, A516-70
- Stress relieved
- Full secondary containment
- Tapered design from 0.5-in to 0.875-in thickness



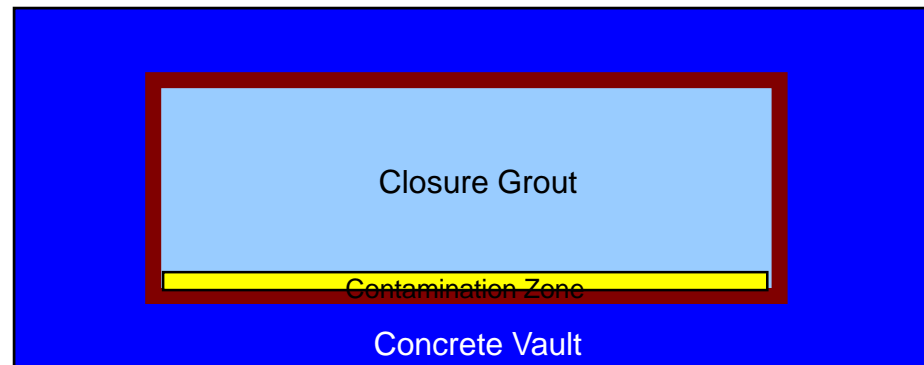
- Type IV Tanks
- Steel-lined prestressed concrete tank
- ASTM A285 Grade B Steel
- 0.375-in thick walls
- 0.4375-in thick bottom
- Vintage 1950s

# Waste Tank Closure

- Closed through bulk waste removal, chemical cleaning, heel removal, stabilizing remaining residuals with tailored grout formulations, and severing/sealing external penetrations
- Performance assessment supporting closure of F-Tank Farm
- Carbon steel of high level waste tank initially provide a barrier contaminant escape
- Corrosion mechanisms will degrade liner over time
- Liner will no longer provide a barrier
- Estimate the time to failure of the tank liner due to corrosion processes

# Life Estimation Methodology

- Goal: Determine time period that steel liners can act as a barrier to contaminant escape
  - Penetrations of tank steel
  - Size of penetrations
- Active corrosion mechanisms on the steel under closure conditions





# Contamination Zone

- Function of the undissolved solids in the residual on tank bottom
- R-value: Ratio of inhibitor species (nitrite and hydroxide) to aggressive species (nitrate + chloride)
  - High R-values: Minimal Corrosion
  - Low R-values High corrosion due to insufficient inhibitors
- Results indicate no accelerated corrosion from contamination zone

Tank	R-Value
1	5.47
2	4.36
3	3.94
4	9.67
5	5.31
6	12.39
7	3.44
8	3.87
17	3.18
18	4.51
19	0.24
20	3.18
25	3.19
26	3.19
27	3.19
28	3.19
33	4.53
34	12.40
44	4.45
45	3.19
46	3.19
47	3.19

# Corrosion in Concrete/Grout

- Corrosion of steel exposed to concrete/grout occurs by a complex mechanism that occurs through metal dissolution at the concrete/metal interface.
- Concrete generally prevents corrosion of the steel
  - Forms passive oxide on the steel surface
  - Maintains a high pH environment
  - Provides a matrix resistant to diffusion of aggressive species
- Passivity can be lost through carbonation or through chloride induced film breakdown
  - Pore water characteristics change with the introduction of chlorides or carbon dioxide, the passive film on the steel may break down



# Stochastic Technical Approach

- Proposed to account for potential uncertainty in the time-frames proposed for regulatory compliance
- Initially Considered
  - First order reliability methods (FORM)
    - Statistical information is sparse
    - Marginal probability distributions
  - Direct uncertainty analysis
    - Separation of the probability calculations from the evaluation of the performance measure
    - Discretization of the probability intervals
- Ultimately, USED Monte Carlo Simulation
  - Inherently represent the uncertainties in the deterministic approach
  - Large number of simulations
  - Exploits the in-depth knowledge of SRS subsurface environments and HLW tanks as input distributions for the simulations

# Stochastic Technical Approach

- Life of the tank liners was assumed to be a function of the time to corrosion initiation plus the time for corrosion to propagate through the liner
- Grouted Conditions
  - Corrosion in grouted conditions
  - Chloride induced depassivation, followed by general corrosion
  - Carbonation induced loss of protective capacity of the concrete

$$t_{failure} = t_{initiation} + \frac{Thickness(mils)}{CorrosionRate(mils / year)}$$

$t_{failure}$	=	time to penetration of the tank wall by corrosion
$t_{initiation}$	=	time to chloride induced depassivation or carbonation front
Thickness	=	initial thickness of liner (mils)
Corrosion rate	=	Dependent upon condition, i.e. chloride or carbonation

## Case 1: IF $t_{\text{initiation}} [\text{Cl}^-] \geq t_{\text{initiation}} [\text{Carbonation}]$

$$t_{\text{failure}} = t_{\text{initiation}[\text{carbonation}]} + \frac{\text{Thickness}(\text{mils})}{\text{CorrosionRate}(\text{mils} / \text{year})}$$

$T_0$	=	Initial Thickness (mils)
Thickness	=	$T_0 - \text{RNDCR} * t_{\text{init}[\text{carbonation}]} [\text{mils}]$
RNDCR	=	Corrosion Rate per Distribution
Corrosion Rate (Rcarb)	=	10 mils/year

$$t_{\text{failure}} = t_{\text{initiation}[\text{carbonation}]} + \frac{T_0 - \left( \text{RNDCR} \left( \frac{\text{mils}}{\text{yr}} \right) \cdot t_{\text{initiation}[\text{carbonation}]} \right)}{10 \left( \frac{\text{mils}}{\text{yr}} \right)}$$



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## Case 2: IF $t_{\text{initiation}} [\text{Cl}^-] < t_{\text{initiation}} [\text{Carbonation}]$

$$t_{\text{failure}} = t_{\text{initiation}[\text{chloride}]} + \frac{\text{Thickness}(\text{mils})}{\text{CorrosionRate}(\text{mils} / \text{year})}$$

$T_0$  = Initial Thickness (mils)  
Thickness =  $T_0 - \text{RNDCR} * t_{\text{init}[\text{chloride}]}$  [mils]  
Corrosion Rate ( $R_{\text{Cl}^-}$ ) = Calculated

## Embedded Case 3: IF $t_{\text{failure}} [\text{Cl}^-] \geq t_{\text{initiation}} [\text{Carbonation}]$

$$t_{\text{failure}} = t_{\text{initiation}[\text{chloride}]} + \frac{\text{Thickness(mils)}}{\text{CorrosionRate(mils/year)}}$$

$T_0$  (mils) = Initial Thickness

Thickness =  $T_0 - \text{RNDCR} \cdot t_{\text{init}[\text{carbonation}]} [\text{mils}]$

Corrosion Rate (R) = Dependent upon Reaction

$$T_0 - \left[ (t_{\text{initiation}[\text{carbonation}]} - t_{\text{initiation}[\text{Cl}^-]}) \cdot R_{\text{Cl}} + (t_{\text{initiation}[\text{Cl}^-]} \cdot \text{RNDCR}) \right]$$

$$t_{\text{failure}} = t_{\text{initiation}[\text{carbonation}]} + \frac{T_0 - \left[ (t_{\text{initiation}[\text{carbonation}]} - t_{\text{initiation}[\text{Cl}^-]}) \cdot R_{\text{Cl}} + (t_{\text{initiation}[\text{Cl}^-]} \cdot 0.04) \right] (\text{mils})}{10 (\text{mils/year})}$$

# Chloride Induced Corrosion: Initiation

- Due to the breakdown of the passive film, thereby indicating that chloride diffusion is the rate controlling step for corrosion initiation
- Followed by oxygen diffusion for corrosion reactions to occur

- Simple Empirical Model:

$$t_{\text{initiation}} = \frac{129 \cdot t_c^{1.22}}{WCR \cdot [Cl^-]^{0.42}}$$

$t_{\text{initiation}}$  = time required for initiation  
(years)

$t_c$  = thickness of the concrete  
cover (in.)

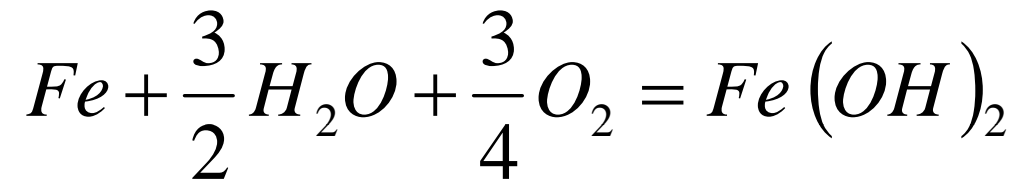
WCR = water-to-cement ratio

$[Cl^-]$  = chloride concentration in the  
groundwater (ppm)



# Chloride Induced Corrosion: Reaction

- Oxygen diffusion to breakdown of passivity: corrosion reaction



- Corrosion rate

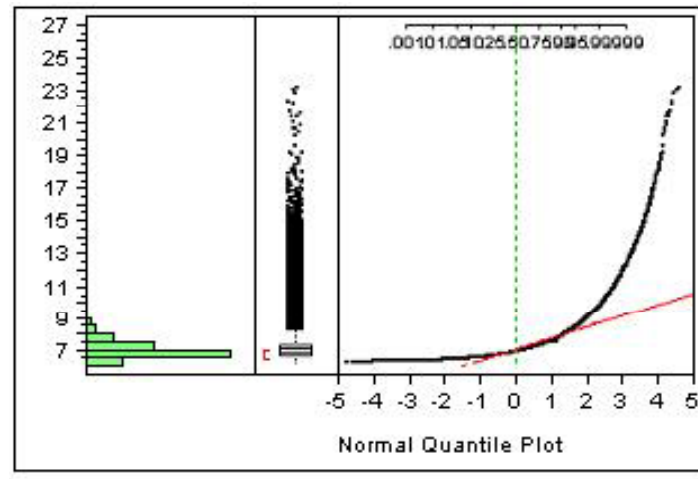
$$R_{corrosion} = \frac{4}{3} N_{O_2} \frac{M_{Fe}}{\rho_{Fe}}$$

$M_{Fe}$  = molecular weight of iron (56 g/mol)  
 $\rho_{Fe}$  = density of iron (7.86 g/cm<sup>3</sup>)

$$N_{O_2} = D_i \frac{C_{gw}}{\Delta X}$$

$N_{O_2}$  = Flux of oxygen through concrete (mol/s/cm<sup>2</sup>)  
 $D_i$  = Oxygen diffusion coefficient in concrete (cm<sup>2</sup>/sec)  
 $C_{gw}$  = Concentration of oxygen in groundwater (mol/cm<sup>3</sup>)  
 $\Delta X$  = Depth of concrete (cm)

# Chloride Distribution



100.0%	maximum	26.893
99.5%		10.365
97.5%		8.875
90.0%		7.846
75.0%	quartile	7.269
50.0%	median	6.866
25.0%	quartile	6.619
10.0%		6.480
2.5%		6.382
0.5%		6.327
0.0%	minimum	6.249

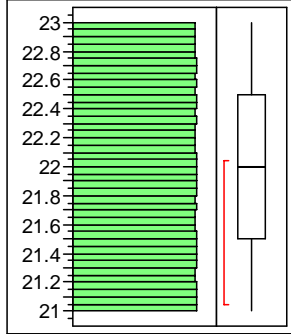
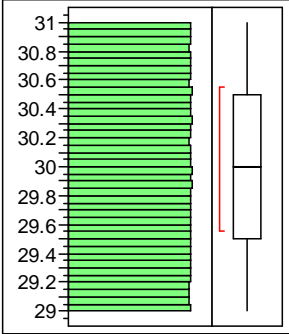
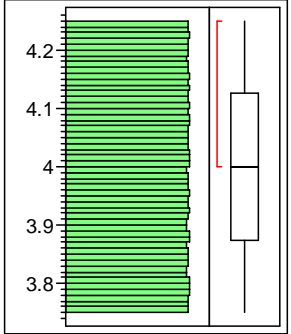
# Carbonation

- Pore water pH reduces dramatically due to the conversion of the calcium hydroxide to calcium carbonate through reaction with carbon dioxide
- Complex function of the permeability of the concrete, relative humidity, and the carbon dioxide availability

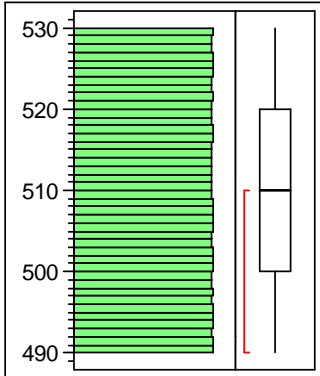
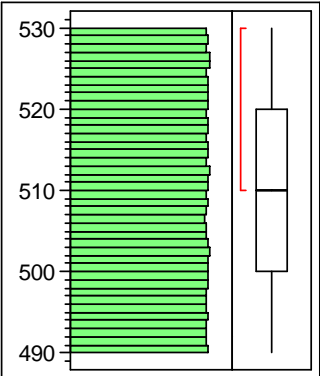
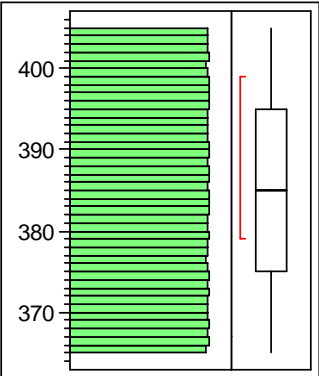
$$t_{\text{initiation[carbonation]}} = \frac{X^2 C_g}{D_{i(\text{CO}_2)} C_{\text{gw(carbon)}}$$

X	= carbonation depth (cm)
$D_i$	= diffusion coefficient of $\text{CO}_2$ in concrete ( $\text{cm}^2/\text{s}$ )
$C_{\text{gw(carbon)}}$	= total inorganic carbon ( $\text{mol}/\text{cm}^3$ )
$C_g$	= $\text{Ca}(\text{OH})_2$ bulk concentration ( $\text{mole}/\text{cm}^3$ )
T	= time (years)

# Concrete Thickness Input

Distribution of Type I Tank Concrete Thickness (in.)			Distribution of Type III Tank Concrete Thickness (in.)			Distribution of Type IV Tank Concrete Thickness (in.)		
								
<b>Quantiles</b>			<b>Quantiles</b>			<b>Quantiles</b>		
100.0%	maximum	23.000	100.0%	maximum	31.000	100.0%	maximum	4.2500
99.5%		22.990	99.5%		30.990	99.5%		4.2475
97.5%		22.950	97.5%		30.950	97.5%		4.2375
90.0%		22.799	90.0%		30.800	90.0%		4.2001
75.0%	quartile	22.499	75.0%	quartile	30.500	75.0%	quartile	4.1251
50.0%	median	21.998	50.0%	median	30.000	50.0%	median	4.0005
25.0%	quartile	21.499	25.0%	quartile	29.502	25.0%	quartile	3.8751
10.0%		21.199	10.0%		29.201	10.0%		3.7998
2.5%		21.050	2.5%		29.050	2.5%		3.7625
0.5%		21.010	0.5%		29.010	0.5%		3.7525
0.0%	minimum	21.000	0.0%	minimum	29.000	0.0%	minimum	3.7500
<b>Moments</b>			<b>Moments</b>			<b>Moments</b>		
Mean		21.999208	Mean		30.000481	Mean		4.0001247
Std Dev		0.577186	Std Dev		0.5769515	Std Dev		0.1443445
Std Err Mean		0.0005772	Std Err Mean		0.000577	Std Err Mean		0.0001443
upper 95% Mean		22.00034	upper 95% Mean		30.001612	upper 95% Mean		4.0004076
lower 95% Mean		21.998077	lower 95% Mean		29.99935	lower 95% Mean		3.9998418
N		1000000	N		1000000	N		1000000

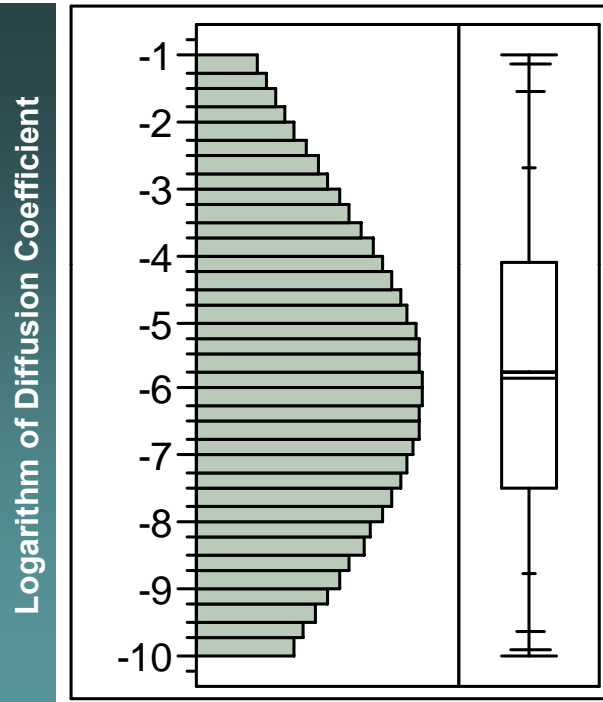
# Tank Steel Thickness Input

Distribution of Type I Tank Steel Thickness (mils)			Distribution of Type III Tank Steel Thickness (mils)			Distribution of Type IV Tank Steel Thickness (mils)		
								
<b>Quantiles</b>			<b>Quantiles</b>			<b>Quantiles</b>		
100.0%	maximum	530.00	100.0%	maximum	530.00	100.0%	maximum	405.00
99.5%		529.80	99.5%		529.80	99.5%		404.80
97.5%		529.00	97.5%		529.00	97.5%		404.00
90.0%		526.00	90.0%		526.01	90.0%		401.00
75.0%	quartile	520.01	75.0%	quartile	520.04	75.0%	quartile	395.00
50.0%	median	509.98	50.0%	median	510.05	50.0%	median	385.01
25.0%	quartile	500.00	25.0%	quartile	500.03	25.0%	quartile	374.99
10.0%		494.00	10.0%		494.02	10.0%		369.00
2.5%		491.00	2.5%		491.00	2.5%		366.02
0.5%		490.20	0.5%		490.20	0.5%		365.21
0.0%	minimum	490.00	0.0%	minimum	490.00	0.0%	minimum	365.00
<b>Moments</b>			<b>Moments</b>			<b>Moments</b>		
Mean		509.99614	Mean		510.02181	Mean		385.00467
Std Dev		11.550194	Std Dev		11.54662	Std Dev		11.542438
Std Err Mean		0.0115502	Std Err Mean		0.0115466	Std Err Mean		0.0115424
upper 95% Mean		510.01878	upper 95% Mean		510.04444	upper 95% Mean		385.02729
lower 95% Mean		509.97351	lower 95% Mean		509.99918	lower 95% Mean		384.98205
N		1000000	N		1000000	N		1000000



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# Diffusion Coefficient Input



quantiles		
100.0%	max	0.10000
99.5%		0.07509
97.5%		0.02822
90.0%		0.00215
75.0%	quartile	0.00008
50.0%	median	1.47e-6
25.0%	quartile	3.06e-8
10.0%		1.68e-9
2.5%		2.3e-10
0.5%		1.2e-10
0.0%	min	1e-10

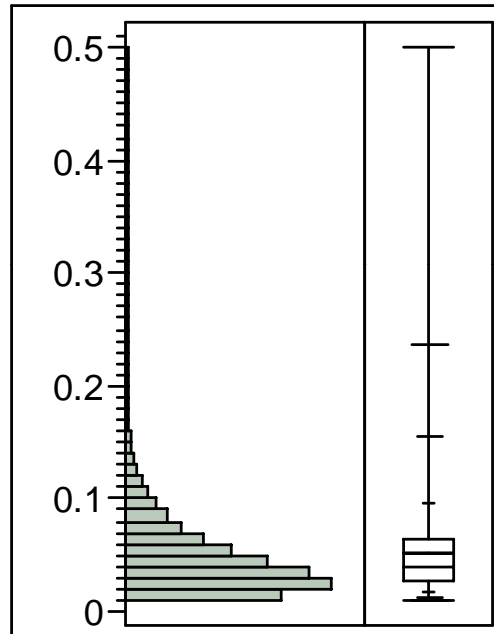




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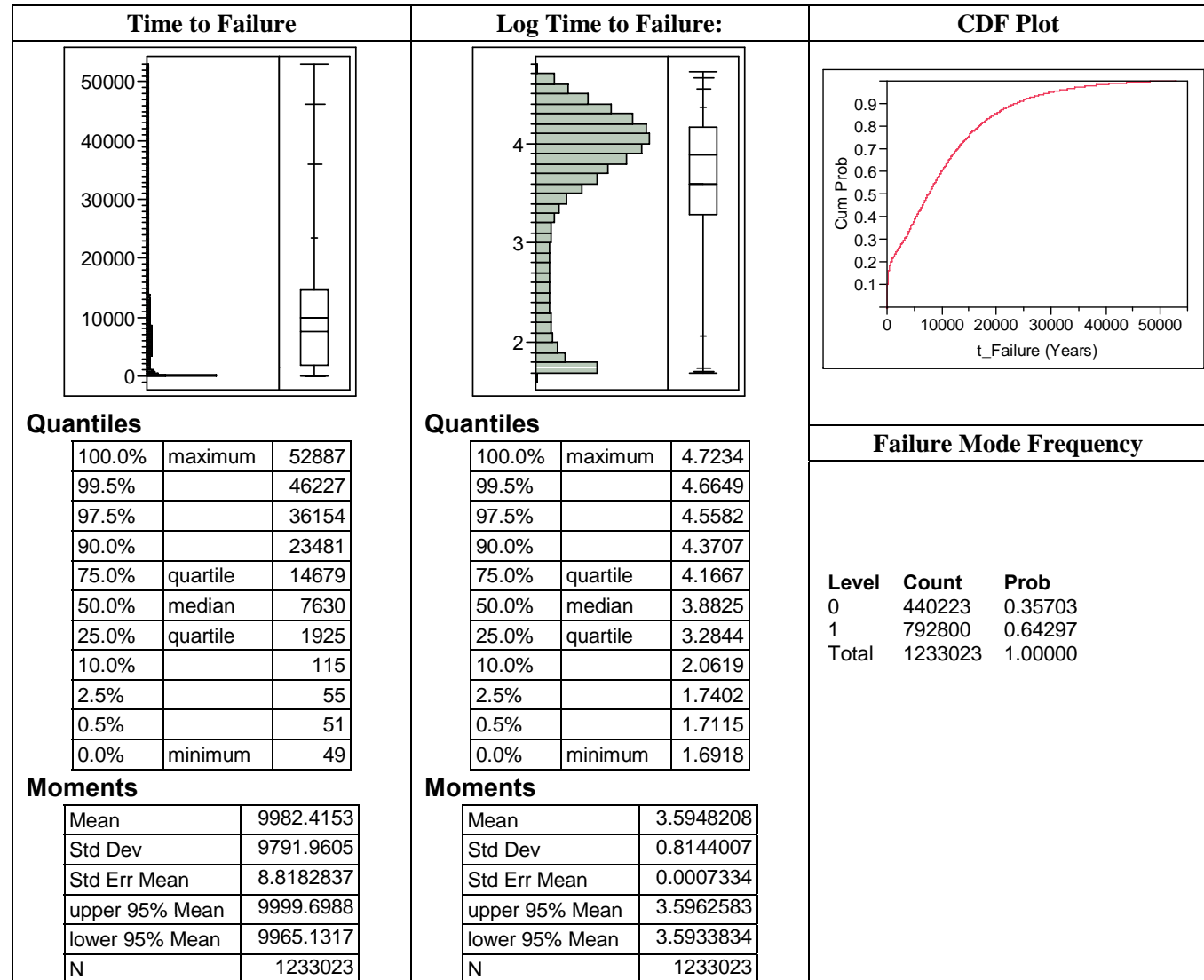
# Corrosion Rate Distribution

Corrosion Rate (mils/year)

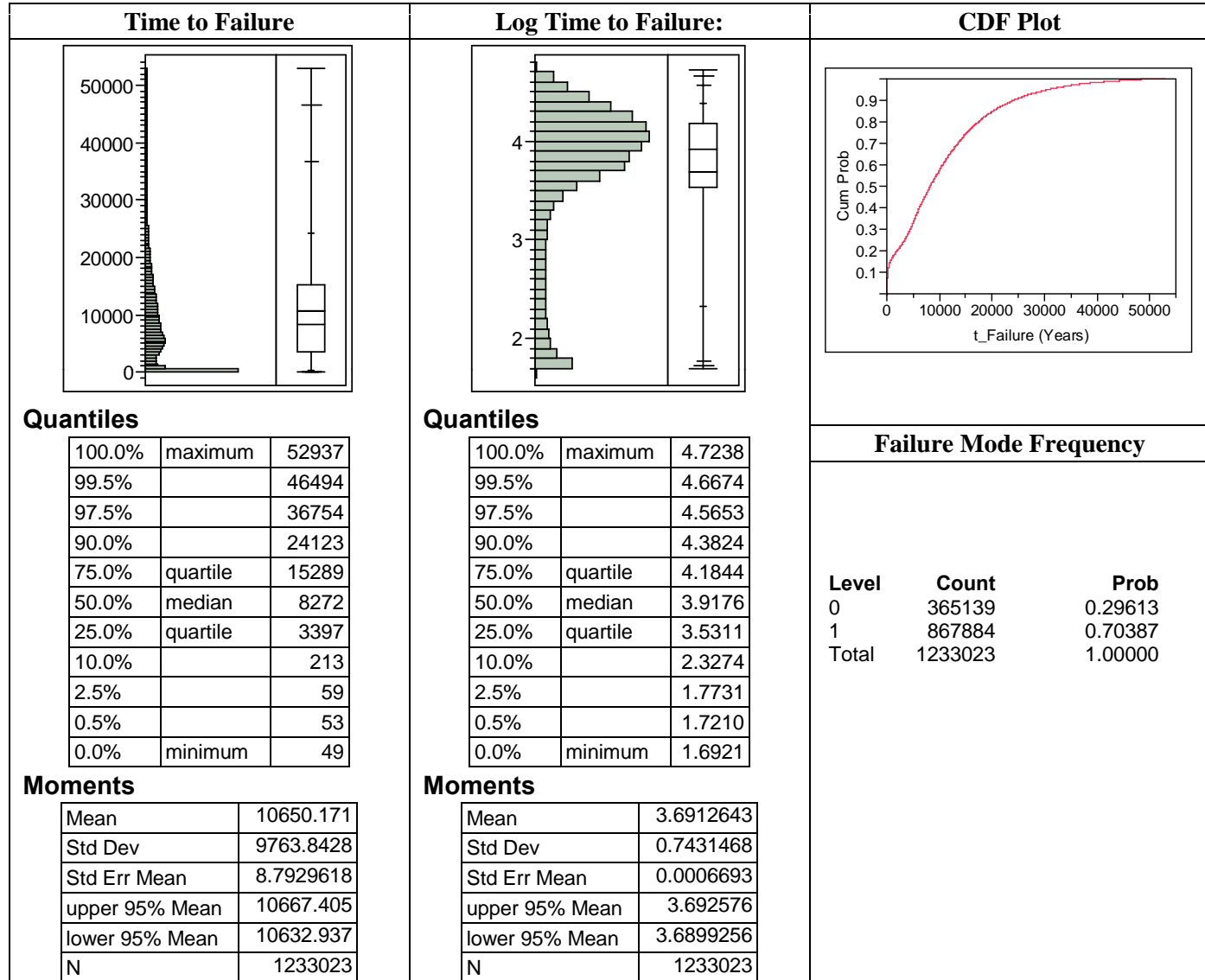


%		mils/yr	$I_{\text{corr}}$ ( $\mu\text{A}/\text{cm}^2$ )
100.0%	Max	0.44978	0.9830
99.5%		0.23641	0.5167
97.5%		.15527	0.3393
90.0%		0.09727	0.2126
75.0%	Quartile	0.06418	0.1402
50.0%	Median	0.04058	0.0886
25.0%	Quartile	0.02590	0.0566
10.0%		0.01769	0.0386
2.5%		0.01253	0.0273
0.5%		0.01059	0.0231
0.0%	Min	0.01	0.0218

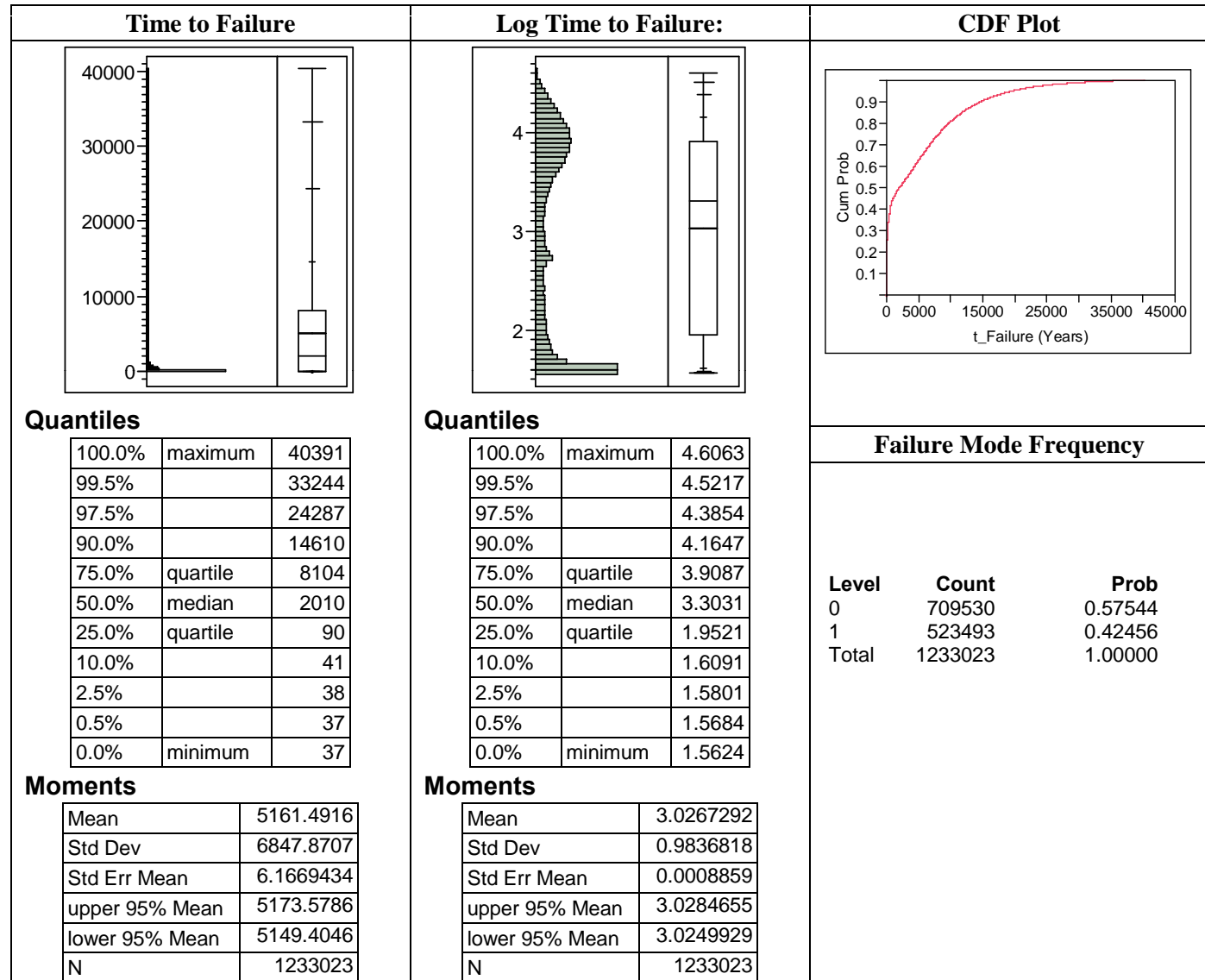
# Type I Monte Carlo Simulation



# Type III/IIIA Monte Carlo Simulation



# Type IV Monte Carlo Simulation



# Recommendations

- The distributions of failure may be used as input for modeling the migration of contaminants via various mechanisms
- Median value as a best estimate for failure times under the assumption of complete consumption
- Figure of merit for percentage breached for a “patch” type models which will progressively fail the tank and assume that past a critical percentage breached, the tank no longer acts as a barrier to contaminant escape
- Entire distribution in any stochastic modeling



# Concluding Remarks

- Unique condition of modeling vintage materials and infrastructural components as compared to design and build
- Utilize best engineering judgment to quantify assumptions
- Questions???